

## 1 Abstract

2 Flooding of settlements is a growing concern in Europe, also in  
3 agricultural areas. Restoration and installation of vegetated landscape  
4 elements (vLE) such as hedges, lines of trees and grass buffers, along  
5 the parcel boundaries is increasingly recognized as a way to mitigate  
6 downstream flood risk. However, there is a lack of scientific evidence to  
7 support their implementation. We used the Landlab modelling  
8 framework to gain knowledge about the importance of the presence  
9 and characteristics of vLEs for the hydrological response in a 26 hectare  
10 undulating watershed representative for the Belgian loess belt for which  
11 a multitude of vLE scenarios were developed. Our model results  
12 demonstrated that the total runoff volume, the peak discharge rate and  
13 its lag time in such small watersheds are mainly controlled by the  
14 density of the vLE objects and their upstream area. First and foremost  
15 we demonstrated a negative correlation between the discharge volume  
16 and peak discharge rate and the density of the vLE objects and their  
17 upstream area. A positive correlation was observed between the lag  
18 time and density of the vLE objects for both dry and wet soils and  
19 between the lag tag time and upstream area for dry soils. Further, we  
20 found that the impact of the value of the saturated hydraulic  
21 conductivity of the soil covered by the vLE became increasingly  
22 important with increasing soil wetness, with the hydraulic conductivity  
23 being negatively correlated with the discharge volume and peak

24 discharge rate. The impact of hydraulic conductivity on the lag time was  
25 limited. A negative correlation between hydraulic conductivity and lag  
26 time for intermediate wet soils was demonstrated. Our model results  
27 also showed that the roughness, expressed as the Manning n-  
28 coefficient, of the soil underneath a vLE and the spatial connectivity of  
29 the vLE objects have little impact on the hydrological response.

30

## Highlights

- Modelling results confirm that landscape elements contribute to lowering flood risk
- Higher initial soil wetness levels result in more and faster discharge
- Runoff is controlled by the density of landscape elements and their upstream area

1    **Title:** The impact of vegetated landscape elements on runoff in a small  
2    agricultural watershed: A modelling study

3    **Authors names and affiliations:**

4    Ine Rosier<sup>A</sup>

5    <sup>A</sup>Department of Earth and Environmental Sciences, KU Leuven,  
6    Celestijnenlaan 200E - bus 2411, B-3001 Leuven, Belgium

7    ine.rosier@kuleuven.be

8    Jan Diels<sup>A</sup>

9    <sup>A</sup>Department of Earth and Environmental Sciences, KU Leuven,  
10   Celestijnenlaan 200E - bus 2411, B-3001 Leuven, Belgium

11   jan.diels@kuleuven.be

12   Ben Somers<sup>A</sup>

13   <sup>A</sup>Department of Earth and Environmental Sciences, KU Leuven,  
14   Celestijnenlaan 200E - bus 2411, B-3001 Leuven, Belgium

15   ben.somers@kuleuven.be

16   Jos Van Orshoven<sup>A</sup>

17   <sup>A</sup>Department of Earth and Environmental Sciences, KU Leuven,  
18   Celestijnenlaan 200E - bus 2411, B-3001 Leuven, Belgium

19   jos.vanorshoven@kuleuven.be

20   **Corresponding author:** Ine Rosier (ine.rosier@kuleuven.be)

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22 landscape element; natural flood management

## 23 1. Introduction

24 Extensive areas throughout Europe are affected by flooding of rivers or  
25 from surface runoff. This can be demonstrated by the destructive  
26 events of July 2021 in Western Europe that caused an estimated loss of  
27 up to €5.8 billion and over 200 deaths (Kreienkamp et al., 2021). In the  
28 period between 1980 and 2013, almost 1500 flood and wet mass  
29 movement events happened within the European Union, more than half  
30 of them since 2000 (EEA, 2017). In Belgium, flood events are a common  
31 occurrence in the Belgian loess belt, which covers about 34 % (10576  
32 km<sup>2</sup>) of the Belgian territory. These flood events are often co-  
33 determined by runoff from agricultural land. Between 1991 and 2004,  
34 79 % of the municipalities located within the Belgian loess belt were  
35 affected by floods resulting from runoff from arable land (Biolders et al.,  
36 2003; Evrard et al., 2007a). Flood hazard is likely to increase in this  
37 region as a result of the expected global-warming related changes in the  
38 frequency and magnitude of extreme precipitation events (Fowler et al.,  
39 2021; Kreienkamp et al., 2021). Climate-smart upstream land use  
40 systems, and hence climate-smart land use planning, are increasingly  
41 recognized as a way to mitigate downstream flood risk (Gabriels et al.,  
42 2022; Minang et al., 2015). Vegetated landscape elements (vLEs) such as  
43 hedges, lines of trees and grass buffers are inherent components of

44 such climate-resilient agricultural land use systems (Burgess-Gamble et  
45 al., 2017; Ellis et al., 2021). Their typical geometrical arrangement  
46 following the edges of agricultural parcels creates networks of  
47 landscape elements. These networks alter the parcel and catchment  
48 hydrology since they create hydrological discontinuities by impeding  
49 flow paths (Mérot, 1999) or enhancing flow continuities exacerbating  
50 runoff. Hence, vLEs alter the runoff pattern and hence affect the  
51 frequency, extent, depth and duration of downstream flood events  
52 (Horn et al., 2007; Klaassen and Zwaard, 1974; Mérot, 1999; Richet et  
53 al., 2017). The attitude of landowners towards these potential natural  
54 flood protection measures is not always positive, which can partly be  
55 explained by the current lack of scientific evidence about their  
56 effectiveness (Bielders et al., 2003; Ellis et al., 2021; Wells et al., 2020).  
57 Still, there is a strong positive correlation between the probability of a  
58 landowner taking flood and erosion control measures (e.g. grass buffer  
59 strips) and the probability of having experienced runoff or erosion  
60 damage during the last decade (Bielders et al., 2003).

61 To design evidence-based climate-smart landscapes, quantitative  
62 information about the effect of vLEs on the runoff in a catchment is  
63 needed. vLEs and their hydrological properties have been the subject of  
64 numerous studies. These studies demonstrate that the effect of the vLE  
65 on the infiltration capacity of the soil covered by that vLE depends on  
66 the type of vLEs with grass buffers decreasing the infiltration capacity  
67 compared to some common crop types while hedges increase the

68 infiltration capacity (Bartman et al., 2020; Holden et al., 2019). Further,  
69 it was shown that vLEs are typically associated with higher hydraulic  
70 roughness values (Bartman et al., 2020; Richet et al., 2017). This not  
71 only results in a decreased velocity of runoff but also in lower sediment  
72 transport. Previously, these studies focused mainly on the impact of  
73 vLEs on runoff at the field scale (Holden et al., 2019; Richet et al., 2017;  
74 Wallace et al., 2021). However, information on these effects at a  
75 catchment scale is at least equally important as many of the off-site  
76 consequences of flooding events have to be managed at a catchment  
77 scale. Also on the catchment scale, it becomes possible to assess the  
78 effect of the geometric characteristics of the vLEs (i.e. dimensions,  
79 position along the concentrated flow paths and connectivity) on runoff  
80 within a catchment, and to investigate how these characteristics affect  
81 the rainfall-runoff behaviours of the catchment.

82 With this research, we aimed to gain knowledge about the importance  
83 of the presence and characteristics of vLEs on the hydrological response  
84 of a small watershed representative of the Belgian loess belt. We used a  
85 physically-based distributed rainfall-runoff model implemented in the  
86 Landlab modelling framework (Barnhart et al., 2020; Hobbey et al., 2017)  
87 to quantify the effect of various configurations and characteristics of  
88 vLEs using a design storm (Willems, 2013). Our findings are meant to aid  
89 in the conservation and promotion of vLEs in an agricultural landscape  
90 in the future. This is important as shown by the declining trend in the  
91 presence of vLEs demonstrated by the disappearance of more than half

92 of the hedgerows between 1900 and 2002 in Flanders (Deckers et al.,  
93 2005).

94 The specific objectives of this research were:

- 95 1. To assess the sensitivity of the hydrological modelling  
96 framework Landlab (Barnhart et al., 2020; Hobbey et al., 2017)  
97 to changes in the hydrological properties of vLEs (i.e. Manning's  
98 roughness coefficient ( $n$ ) and saturated hydraulic conductivity  
99 of the underlying soil ( $K_s$ ));
- 100 2. To compare the magnitude and timing of the flood peak  
101 discharge rate and the total runoff volume produced by a design  
102 storm between various configurations of vLEs;
- 103 3. To assess the impact of contrasting initial soil moisture  
104 contents on the hydrological functioning of vLEs.

105

## 106 2. Methodology

### 107 2.1. Rainfall-runoff model

108 The open-source Python-coded Landlab modelling framework (Barnhart  
109 et al., 2020; Hobbey et al., 2017) was used to simulate overland flow and  
110 infiltration in a real watershed with assumed vLEs. This modelling  
111 framework has previously been used and validated to model catchment  
112 runoff (Adams et al., 2017; Reitman et al., 2019; Zhang et al., 2020).

113 Overland flow in the Landlab modelling framework is based on the two-  
114 dimensional shallow water equations (SWE) as is the case for many  
115 physically-based hydrological models (e.g. Cea and Bladé, 2015; Defina,



116 2000; Warnock et al., 2014). The SWE are a simplification of the Navier-  
 117 Stokes equations in the vertical direction and consist of two parts, the  
 118 conservation of mass (Eq. 1) and the conservation of momentum (Eq. 2  
 119 and Eq. 3):

$$\frac{\partial h}{\partial t} + \frac{\partial q_x}{\partial x} + \frac{\partial q_y}{\partial y} = 0 \quad (1)$$

$$\underbrace{\frac{\partial q_x}{\partial t}}_{\text{local acceleration}} + \underbrace{\frac{\partial}{\partial x}(uq_x) + \frac{\partial}{\partial y}(vq_x)}_{\text{convective acceleration}} +$$

(2)

$$\underbrace{gh \frac{\partial(h+z)}{\partial x}}_{\text{pressure+ bed gradients}} + \underbrace{\frac{gn^2 \|q\| q_x}{h^{7/3}}}_{\text{friction}} = 0$$

$$\underbrace{\frac{\partial q_y}{\partial t}}_{\text{local acceleration}} + \underbrace{\frac{\partial}{\partial y}(uq_y) + \frac{\partial}{\partial x}(vq_y)}_{\text{convective acceleration}} + \underbrace{gh \frac{\partial(h+z)}{\partial y}}_{\text{pressure+ bed gradients}} = 0$$

(3)

$$\underbrace{\frac{gn^2 \|q\| q_y}{h^{7/3}}}_{\text{friction}} = 0$$

120 where  $x$  [m] and  $y$  [m] are the planimetric Cartesian directions,  $t$  [s] is  
 121 time,  $h$  [m] is water depth,  $q_x$  and  $q_y$  [ $\text{m}^2 \text{s}^{-1}$ ] are the  $x$  and  $y$  components  
 122 of the discharge per unit width vector  $\mathbf{q}$ ,  $(u, v)$  [ $\text{m s}^{-1}$ ] are the velocities  
 123 at  $x, y$ -direction,  $z$  [m] is the bed elevation,  $g$  [ $\text{m s}^{-2}$ ] is the gravity  
 124 acceleration, and  $n$  [ $\text{s m}^{-1/3}$ ] is the Manning's  $n$ . Since full Shallow Water  
 125 models are computationally expensive, some studies suggest  
 126 approximating or omitting specific terms in the SWE (e.g. Bates et al.,  
 127 2010; Singh, 1997). We used LISFLOOD-FP, a simplified approximation of  
 128 the SWE that omits the convective acceleration term in Eq. 2 and Eq. 3  
 129 (Bates et al., 2010; Bates and De Roo, 2000; de Almeida et al., 2012; de  
 130 Almeida and Bates, 2013). The original python code from the Landlab

131 library modelling framework was adapted to allow defining a unique  
132 Manning's  $n$  for each cell in the gridded watershed. This roughness  
133 coefficient was used to drive overland flow. The flow direction is  
134 determined by defining for each cell the steepest path in the four  
135 cardinal directions. The outlet was selected to be the only location in  
136 the watershed where water can exit the watershed. Therefore, all other  
137 boundary cells were set as 'no flux' cells.

138 For each time step, water losses due to infiltration were calculated after  
139 flow was routed by using the Green-Ampt Mein-Larson infiltration  
140 model (GAML) (Mein and Larson, 1971). GAML describes the infiltration  
141 rate as:

$$f = K_e \left( 1 + \frac{\psi \Delta \theta}{F} \right) \quad (4)$$

142 where  $f$  [ $\text{m s}^{-1}$ ] is the potential infiltration rate,  $F$  [ $\text{m}$ ] is the cumulative  
143 infiltration,  $\psi$  [ $\text{m}$ ] is the capillary pressure head at the wetting front,  $\Delta \theta$   
144 [ $\text{m}^3 \text{m}^{-3}$ ] is the difference between saturated and initial volumetric  
145 moisture content, and  $K_e$  [ $\text{m s}^{-1}$ ] is the effective hydraulic conductivity.  
146  $K_e$  is a lumped parameter that adjusts  $K_s$  to account for spatial variation  
147 in rainfall intensity and soil properties (e.g., soil crusting, surface  
148 microtopography and soil pore structure) (Langhans et al., 2010b; Van  
149 den Putte et al., 2013). The GAML describes a situation where runoff  
150 occurs only after some time, i.e. the ponding time. A minimum water  
151 depth on the surface of  $1.0\text{E-}8$  m that can not infiltrate was assumed.  
152 This was done to avoid numerical instability of the solutions of the

153 overland flow modelling resulting from the calculation of negative water  
154 depths, as suggested by Costabile et al. (2012).

## 155 2.2. Model construction

### 156 2.2.1. *Study area*

157 A 26 ha agricultural watershed situated in the Belgian loess belt was  
158 selected (50.72° N, 5.12°E). This area has been repeatedly affected by  
159 floods as happens frequently in undulating agricultural areas on loamy  
160 soils in Flanders (Biielders et al., 2003; Evrard et al., 2007b). The  
161 elevation in the watershed was characterized by a 2 m resolution digital  
162 elevation model (DEM). Altitudes range between 106 m and 120 m  
163 above sea level. The majority of the land cover (94 %) in the study area  
164 is agricultural land (i.e. arable land and agricultural grassland) (ALV,  
165 2021).

166 [Figure 1]

### 167 2.2.2. *Vegetation cover*

168 Existing field boundary patterns were used to generate different  
169 configurations of vLEs. The first three patterns (Figure 2, a-c) occur  
170 elsewhere in Belgium and were chosen to be representative of a typical  
171 agricultural field pattern (FP) in the region. These three patterns were  
172 selected by assessing the parcel configuration around a selection of  
173 1000 randomly chosen agricultural fields in the region and selecting a  
174 field pattern with a small, medium and large average plot size. FP1  
175 (Figure 2, a) is characterized by smaller plots with an average field size  
176 of 1.42 ha. FP2 (Figure 2, b) is characterized by medium-sized fields with

177 an average size of 1.89 ha. FP3 (Figure 2, c) is characterized by larger  
178 fields with an average field size of 2.33 ha. The fourth pattern (FP4,  
179 Figure 2, d) was generated to have field boundaries perpendicular and  
180 parallel to the flow directions in the watershed.

181 [Figure 2]

182 In total, 42 configurations of vLEs were created based on the field  
183 boundaries of FP1, FP2, FP3 and FP4. These configurations differ in the  
184 density of vLEs, connectivity, and upslope area. Three different values of  
185 vLE density were used: 87 m ha<sup>-1</sup>, 40 m ha<sup>-1</sup> and 10 m ha<sup>-1</sup>. The highest  
186 vLE density represents the mean density of vLEs in Flanders in the year  
187 1900, while a density of 40 m ha<sup>-1</sup> represents the average situation in  
188 2002 (Deckers et al., 2005). A watershed with a vLE density of 10 m ha<sup>-1</sup>  
189 represents a situation in which the density of vLEs in the landscape is  
190 further reduced, e.g. due to further intensification and heavy machinery  
191 use. For FP1, FP2 and FP3, four configurations per density level, with a  
192 range in connectivity, were generated. To do this, 10000 random  
193 combinations of field borders were selected and the beta connectivity  
194 index ( $\beta$ ) was calculated as:

$$\beta = \frac{e}{v} \quad (5)$$

195 with e the number of vLE segments and v the number of vLE nodes. A  
196 segment was defined as a side of an agricultural field boundary with the  
197 start and end point of the segment being defined as nodes. In case a  
198 start or end node of another segment was positioned along the

199 segment, the segment was split in two at the location of that node. The  
200 two configurations of field borders with the highest  $\beta$  per density level  
201 were then selected. Disconnections in these two configurations were  
202 created by rotating per vLE section of 60 m, the middle 20 m with an  
203 angle of 90 degrees. This was done to assess the impact of the  
204 connectivity of vLEs without changing the density or geographical  
205 position of the vLEs. For FP4, two configurations per density level were  
206 created, one where the vLEs were mainly positioned along the flow  
207 direction in the watershed and one where they were located  
208 perpendicular to the modelled flow direction. While in FP1, FP2 and FP3  
209 we aimed to assess the impact of vLE connectivity on runoff, we focused  
210 on vLE configurations positioned along with or perpendicular to the  
211 main slope in FP4. The vLE configurations were rasterized by assigning  
212 the land use class 'vLE' to all pixels intersected by a vLE field border. All  
213 other pixels in the watershed were considered to be 'landscape' pixels.  
214 The average number of 'vLE' pixels in the watershed was 166, 659 and  
215 1400 for a density level of 10 m ha<sup>-1</sup>, 40 m ha<sup>-1</sup> and 87 m ha<sup>-1</sup>  
216 respectively. This corresponds to 0.29 %, 1.14 % and 2.34 % of the total  
217 number of pixels in the watershed. For all 42 vLE configurations, the  $\beta$ -  
218 index (Eq. 5) and the upslope area per meter vLE were calculated. The  
219 average upslope area per meter vLE was calculated by assessing for  
220 each vLE pixel the size of the area that directly contributes water to that  
221 pixel. The sum of that area for all vLE pixels was then divided by the  
222 total length of the vLE objects in the watershed to obtain the average

223 upslope area per meter 'vLE'. This gives an estimation of the runoff  
224 going through the vLEs in the watershed, with larger values indicating a  
225 larger proportion of the runoff that flows through the vLE.

226 GAML parameters, Manning's  $n$  and other soil parameters were  
227 selected based on values found in literature and are summarized in  
228 Table 1. We assumed that our watershed had a uniform loamy soil. The  
229  $K_s$ , initial volumetric moisture content ( $\vartheta_i$ ) and capillary pressure head at  
230 wetting front were based on values derived by Van den Putte et al.  
231 (2013) from a dataset consisting of 350 rainfall experiments carried out  
232 on 21 arable fields in the Belgian loess belt. The  $K_s$  was equal to 19.2  
233  $\text{mm hr}^{-1}$ , which was the average effective hydraulic conductivity derived  
234 for rainfall experiments carried out in the summer period. The  $\vartheta_i$  was set  
235 to a value between  $0.02 \text{ cm}^3 \text{ cm}^{-3}$  and  $0.29 \text{ cm}^3 \text{ cm}^{-3}$ , which are the  
236 minimum and maximum  $\vartheta_i$  values measured for experiments carried out  
237 in the summer period. Further, a  $\vartheta_i$  of  $0.155 \text{ cm}^3 \text{ cm}^{-3}$  was used as a  
238 medium value of  $\vartheta_i$ . The capillary pressure head at wetting front used in  
239 our model equals 172.7 mm, which was the average value derived for  
240 rainfall experiments carried out in the summer period. The Manning's  $n$   
241 was set to  $0.08 \text{ s m}^{-1/3}$ . This value was based on the value measured by  
242 Takken et al. (1999) for land with the maize crop. Maize was selected as  
243 the only land cover type. Maize is the crop type with the largest spatial  
244 extent in Flanders (ALV, 2021).

245 The vLEs were accounted for in the hydrological model by assigning  
246 adjusted values of  $K_s$  and of Manning's  $n$  to vLE pixels. Three different

247 values for  $K_s$  for the soil under the vLEs were selected. The first value  
248 was equal to  $102.4 \text{ mm hr}^{-1}$  which is representative of loamy soils under  
249 hedges (Holden et al., 2019). The second value was equal to  $20 \text{ mm hr}^{-1}$   
250 which is representative of loamy soils under grass buffers (Bartman et  
251 al., 2020; Evrard et al., 2009). As a third value, an intermediate value  
252 was chosen, i.e.  $51.2 \text{ mm hr}^{-1}$ . Further, also three values of the  
253 Manning's  $n$  linked with vLEs were selected. We used  $0.43 \text{ s m}^{-1/3}$  and  
254  $0.55 \text{ s m}^{-1/3}$ , which are the minimum and maximum values of Manning's  
255  $n$  for hedgerows calculated by Richet et al. (2017). Further, we also used  
256 a Manning's  $n$  of  $0.30 \text{ s m}^{-1/3}$  which is the roughness coefficient  
257 associated with grass buffers (Bartman et al., 2020).

258

### 259 *2.2.3. Precipitation data*

260 This study uses a design storm, i.e. a hypothetical rainfall event  
261 associated with a specific duration and return period, as precipitation  
262 input in the model. Such design storms are often used for flood risk  
263 assessments (Jiang et al., 2019). The 2-hour spatially uniform design  
264 storm with a 50-year return period used in this study was calculated  
265 based on the intensity-duration-frequency (IDF) relationship derived  
266 from the Uccle 10 min rainfall time series (Demarée, 2003) for the  
267 period 1898-2007 (Willems, 2013). The calculation was done by using a  
268 frequency-based method that constructs the hyetograph from the  
269 entire IDF curve (Krvavica and Rubinić, 2020). The IDF relationship was  
270 adjusted to account for multidecadal climate oscillations (Willems,

271 2013). The peak intensity of the rainfall event was equal to 127.87 mm  
272  $\text{hr}^{-1}$  while the total rainfall volume over the catchment in the considered  
273 2-hour period was around 11000  $\text{m}^3$ .

### 274 2.3. Model application

275 The rainfall-runoff model described in section 2.1 was applied for 378  
276 vLE scenarios using Python (Version: 3.9.7). These scenarios were  
277 derived by combining varying landscape patterns, different levels of vLE  
278 density, connectivity, and values of  $K_s$  and Manning's  $n$  associated with  
279 the vLE objects (Appendix A). The 378 scenarios were run for  $\theta_i$  equal to  
280  $0.02 \text{ m}^3 \text{ m}^{-3}$   $0.155 \text{ m}^3 \text{ m}^{-3}$  and  $0.29 \text{ m}^3 \text{ m}^{-3}$ , hereafter referred to dry,  
281 intermediate wet, and wet soils. While the 50-year return period design  
282 storm had a length of 2 hours, the total model run time was set for an  
283 additional 2 hours after rainfall ceased to allow all runoff water to either  
284 reach the outlet of the watershed or infiltrate. After each model run,  
285 the total discharge volume (in  $\text{m}^3$ ), the peak discharge rate at the outlet  
286 (in  $\text{m}^3$  per second) and the lag time between the rainfall and discharge  
287 peaks (in seconds) were derived from the discharge time series.

### 288 2.4. vLE feature impact on runoff

289 The impact of the vLE features on the total discharge volume, the peak  
290 discharge rate and the lag time was assessed by comparing the  
291 differences in the output variables between different values of the vLE  
292 features in a multidimensional analysis on the one hand and in a one-  
293 dimensional analysis on the other hand. All statistical analyses were  
294 done using R software (Version: 4.1.2).



295 *2.4.1. Multidimensional analysis*

296 To take into account feature interactions and rank the importance of  
297 the impact of each considered vLE feature (density, connectivity,  
298 upslope area, Manning's  $n$  and  $K_s$ ) on the output variables, 9 random  
299 forest analyses were conducted, one for each combination of output  
300 variable (3) and soil wetness level (3) using the "randomForest" package  
301 (Version: 4.7-1). A randomly selected bootstrapped sample of 70 % of  
302 the 378 scenarios was used as a training set to fit 500 classification  
303 trees. The remaining 30 % of the observations were used as a testing set  
304 to evaluate the predictive performance of the models. The relative  
305 importance of the features within the model was determined to show  
306 the impact of each input feature on the total discharge volume, the  
307 peak discharge rate and the lag time. This was done by constructing the  
308 global variable importance curves using the package "vivo" (Version:  
309 0.2.1) and "DALEX" (Version: 2.4.1) and comparing the variable  
310 importance of each feature with the maximal variable importance in the  
311 model. Regression trees have previously proven to be successful to  
312 assess the relative impact of features (e.g., Poncelet et al., 2017).

313 *2.4.2. One-dimensional analysis*

314 For each unique combination of density, Manning's  $n$  and  $K_s$ , 14 vLE  
315 configurations were created (Appendix A). For these 14 configurations,  
316 the upslope area and connectivity were derived. For each of the vLE  
317 features (density, Manning's  $n$  and  $K_s$ , connectivity, upslope area), a  
318 Kruskal-Wallis nonparametric test (Kruskal and Wallis, 1952) was used  
319 to evaluate whether at least one level of these features performed

320 significantly different from the others using the package “stats”  
321 (Version: 4.2.1). The impact of a feature on an output variable was  
322 considered significant if the P-value returned by the test was lower than  
323 0.05. If the impact of the feature was considered significant, a  
324 subsequent post-hoc Dunn’s test with a Bonferroni correction was  
325 performed using the package “dunn.test” (Version: 1.3.5) to determine  
326 which levels of the features differ from each other. These differences  
327 were considered significant if the P-value returned by the test was  
328 lower than 0.05.

### 329 **3. Results**

330 Runoff simulations for the 378 scenarios at three different wetness  
331 levels resulted in 1134 hydrographs from which the discharge volume,  
332 peak discharge rate and lag time were derived. Initial soil moisture  
333 content was shown to have a large impact on the discharge volume,  
334 peak discharge rate and lag time. The average modelled discharge  
335 volume for all 378 scenarios was 3.08 m<sup>3</sup>, 17.01 m<sup>3</sup> and 110.66 m<sup>3</sup> for  
336 dry, intermediate wet, and wet soil respectively. The average modelled  
337 peak discharge rate was 0.01 m<sup>3</sup> s<sup>-1</sup>, 0.02 m<sup>3</sup> s<sup>-1</sup> and 0.08 m<sup>3</sup> s<sup>-1</sup>  
338 respectively, and the average modelled lag time was 360.32 s, 593.62 s  
339 and 593.17 s respectively. For dry soils, discharge in the watershed is  
340 dominated by runoff from a relatively small area close to the outlet as a  
341 large portion of the runoff from areas further away from the outlet can  
342 infiltrate and will not reach the outlet. This results in a discharge peak  
343 that closely follows the peak in rainfall. For wet soils, however, a smaller

344 proportion of the precipitation will infiltrate and more runoff will reach  
345 the outlet of the watershed. Runoff from areas further away from the  
346 outlet has longer travel times and therefore the peak in discharge will  
347 arrive later in time for wet soils. An illustration of the hydrograph of  
348 four distinct scenarios for wet soils with the associated hyetograph is  
349 given in Figure 3. The bimodal shape of the hydrographs is the result of  
350 different flow paths arriving at the outlet. The flow path conveying  
351 water from an area close to the outlet of the watershed results in a  
352 peak in discharge close to the peak in the rainfall. The flow path  
353 conveying water from an area at a larger distance from the outlet  
354 results in a peak of discharge later in time. The vLE characteristics of the  
355 four scenarios and their corresponding discharge volume, peak  
356 discharge rate and lag time values are given in Table 2.

357 [Figure 3]

### 358 3.1. Multidimensional analysis

359 We used random forest regression models to take into account feature  
360 interactions and rank the importance of the impact of each considered  
361 feature on the output variables. The regression models explained 98 %  
362 of the variation of the output variable 'discharge volume' for all three  
363 wetness levels, between 96 % and 98 % of the variation of the output  
364 variable 'peak discharge rate' and between 33 % and 84 % of the  
365 variation of the output variable 'lag time' (Table 3). The relatively low  $R^2$   
366 value for wet soils suggests there could be other variables influencing  
367 the lag time after a storm than the variables here considered. Our

368 analysis revealed that the vLE features with the highest impact on the  
369 output variables differ per output variable and soil wetness level (Figure  
370 4). The vLE density in the watershed and the upslope area of the vLE  
371 objects were the two most important features in 7 out of 9 random  
372 forest models. For explaining the total discharge volume and peak  
373 discharge rate in wet soil conditions, the two most important variables  
374 were the  $K_s$  value associated with the vLE object and the upslope area of  
375 the vLE objects. The impact of connectivity of the vLE network on the  
376 total discharge volume and peak discharge rate was limited, but an  
377 increase of relative impact could be observed with increasing values of  
378  $\vartheta_i$ . A higher impact of connectivity of the vLE network on the lag time  
379 was observed, where the impact of the connectivity on the lag time  
380 decreased with increasing values of  $\vartheta_i$ . Increasing soil wetness resulted  
381 in a higher relative impact of the  $K_s$  value associated with the vLE object  
382 on the total discharge volume and the peak discharge rate. This was not  
383 the case when the lag time was evaluated whereby the impact of the  $K_s$   
384 value was limited for wet and dry soils. The Manning's n associated with  
385 the vLE objects had little impact on all three output variables.

386 [Figure 4]

### 387 3.2. One-dimensional analysis

388 A Kruskal-Wallis test was performed to identify whether at least one  
389 level of the considered vLE feature performed significantly different  
390 from the others. This was done for the output variable discharge  
391 volume, peak discharge rate and lag time. The considered vLE features

392 were vLE density, the connectivity of the vLE configuration expressed as  
393  $\beta$ , the upslope area of the vLEs, the  $K_s$  associated with the soil  
394 underneath the vLEs, and the Manning's  $n$  associated with the vLEs. The  
395 results are presented in Table 4.

### 396 *3.2.1. Effect of density of vLE objects*

397 The vLE density had a generally negative impact on the total discharge  
398 volume and peak discharge rate and a positive impact on the lag time:  
399 the larger the vLE density in the watershed, the lower the total  
400 discharge volume and peak discharge rate and the larger the lag time  
401 (Figure 5). These effects could also be seen when the storm  
402 hydrographs for scenarios with different density levels were compared  
403 (Figure 3 and Table 2, scenarios 1, 2, 3). An exception to this trend was  
404 observed when the median lag times between a vLE density of  $10 \text{ m ha}^{-1}$   
405 and of  $40 \text{ m ha}^{-1}$  for wet soils were compared. In this case, the lag time  
406 decreases with increasing vLE density. For all soil wetness levels, the  
407 Kruskal-Wallis test demonstrates that these differences were  
408 statistically significant when the discharge volume and peak discharge  
409 rate were evaluated ( $P\text{-value} < 0.05$ ). These differences were more  
410 prominent with increasing levels of initial soil moisture. For the output  
411 variable 'lag time', no statistically significant differences were found  
412 between different levels of vLE density for intermediate wet soils.  
413 However, for dry and wet soils, statistically significant differences were  
414 found ( $P\text{-value} < 0.05$ ) between different levels of vLE density.

415 [Figure 5]

416 *3.2.2. Effect of connectivity*

417 The connectivity of the vLE network had little impact on the total  
418 discharge volume, peak discharge rate and lag time (Figure 6). A  
419 Kruskal-Wallis test showed only a statistically significant difference  
420 between different levels of connectivity for the total discharge volume  
421 and peak discharge rate for dry soils (Table 4). Contrary to our  
422 expectations, we found that increasing values of the  $\beta$ -index (i.e. higher  
423 connectivity in the vLE network) result in a higher total discharge  
424 volume and higher peak discharge rates. No statistically significant  
425 differences were found between the different levels of connectivity and  
426 the lag time for dry soils, nor for any of the output variables for dry or  
427 intermediate wet soils.

428 [Figure 6]

429 *3.2.3. Effect of upslope area*

430 The upslope area had a statistically significant negative impact on the  
431 total discharge volume and peak discharge rate for dry and wet soils  
432 and a significant positive impact on the lag time for dry soils (Figure 7 &  
433 Table 4). Different levels of upslope area in wet soils were shown to also  
434 have significant differences in lag time values but no uniform trend  
435 could be distinguished. The post-hoc Dunn's test demonstrated that for  
436 the lower levels of upslope area, lag time decreased with increasing  
437 values of upslope area, while the highest level of upslope area was  
438 associated with significantly higher values of the lag time. The effects  
439 can also be observed when the storm hydrographs for scenarios with

440 different density levels are compared (Figure 3 and Table 2, scenarios 1  
441 & 2). While the Kruskal-Wallis test identified significant differences in  
442 the total discharge volume and peak discharge rate between different  
443 levels of upslope area in intermediate wet soils, no clear trend could be  
444 observed. Intermediate wet soils did not show any significant  
445 differences in lag time between different levels of upslope area.

446 [Figure 7]

#### 447 *3.2.4. Effect of saturated hydraulic conductivity*

448 A higher  $K_s$  associated with the soil underneath the vLE objects had a  
449 generally negative impact on the total discharge volume and peak  
450 discharge rate: the larger the  $K_s$  value, the lower the total discharge  
451 volume and peak discharge rate (Figure 8). These effects could also be  
452 seen when the storm hydrographs for scenarios with different levels of  
453  $K_s$  values are compared (Figure 3 and Table 2, scenarios 3 & 4). The  
454 differences in total discharge volume and peak discharge rate between  
455 the different  $K_s$  levels were statistically not significant for dry soils (Table  
456 4). The  $K_s$  value associated with the vLE objects was proven to have a  
457 limited impact on the lag time. Only for intermediate wet soils,  
458 statistically significant differences could be identified for the lag time  
459 when different levels of  $K_s$  were considered, with increasing levels of  $K_s$   
460 resulting in a modest decrease in lag time. The modelled difference  
461 between the median lag time of the lowest and highest level of  $K_s$  was  
462 only 0.04 seconds (Figure 8 & Table 4).

463 [Figure 8]

464 *3.2.5. Effect of Manning's roughness coefficient*  
465 The Kruskal-Wallis tests revealed no statistically significant differences  
466 between the different levels of the Manning's  $n$  and the three output  
467 variables (Figure 9 & Table 4).

468 [Figure 9]

## 469 4. Discussion

### 470 4.1. Model construction

471 We used the distributed rainfall-runoff model, implemented in the  
472 Landlab modelling framework, to quantify the impact of vLEs and their  
473 geometric and hydrological characteristics on runoff in a small  
474 watershed. Obviously, reality was simplified in multiple ways in the  
475 model setup. A spatially uniform rainfall event was assumed while in  
476 reality, rainfall is heterogeneously distributed over the catchment.  
477 Interception of rainfall by vegetation was not taken into account,  
478 neglecting the interception of rainfall that lowers the amount that  
479 reaches the soil surface. These interception losses occur both at the  
480 location of the agricultural land and vLE objects. It was quantified that  
481 hedgerows for example can intercept up to 2.6 mm of a precipitation  
482 event (Herbst et al., 2006). By not taking these interception losses into  
483 account, the rainfall amount that reaches the soil surface was  
484 overestimated. We also assumed that the soil parameters (i.e.,  $\vartheta_i$ ,  
485 capillary pressure head at wetting front,  $K_s$  and Manning's  $n$ ) were  
486 spatially uniform in our watershed. These values were, if available,  
487 based on values found in literature reports about experimental studies



488 done in the same region as our study area. For hedgerows, we did not  
489 find  $K_s$  values measured in the Belgian loam belt in the literature.  
490 Therefore we used a value measured in a loamy soil in northern  
491 England. Field studies usually show a large heterogeneity in soil  
492 wetness, both in the horizontal and vertical dimensions (Merz and Plate,  
493 1997). This spatial heterogeneity can be attributed to a variety of  
494 factors including variations in soil characteristics, topography, and water  
495 routing processes (Merz and Plate, 1997). The presence of vLEs also has  
496 an impact on soil moisture, not only directly underneath the object but  
497 also up to 10 m beyond their peripheries (Wallace et al., 2021). The  
498 overland flow routing and therefore the infiltration was calculated  
499 based on a 2 m resolution DEM. At this spatial resolution, non-random  
500 microtopography-related variations of  $K_s$  (Langhans et al., 2010a) cannot  
501 be accounted for and the model assumes water is uniformly spread over  
502 the pixel. This results in an overestimation of the effective hydraulic  
503 conductivity that is dependent on the inundated fraction of the pixel,  
504 and therefore also in an overestimation of the fraction of the  
505 precipitation that can infiltrate. We calculated an average runoff  
506 coefficient (i.e., the total runoff volume divided by the total  
507 precipitation volume) of  $0.0003 \text{ m}^3 \text{ m}^{-3}$  for  $\vartheta_i = 0.02 \text{ m}^3 \text{ m}^{-3}$ ,  $0.0015$   
508  $\text{m}^3 \text{ m}^{-3}$  for  $\vartheta_i = 0.155 \text{ m}^3 \text{ m}^{-3}$  and  $0.0101 \text{ m}^3 \text{ m}^{-3}$  for  $\vartheta_i = 0.29 \text{ m}^3 \text{ m}^{-3}$ .  
509 These values are slightly lower but in the same range as the findings of  
510 Evrard et al. (2007b) and Evrard et al. (2008) who conducted a  
511 hydrological study near our study area. The slightly lower values of the

512 runoff coefficients we calculated could be explained by an  
513 overestimation of the  $K_s$  values used in this study. Hydraulic conductivity  
514 has been proven to be dependent on water depth and rainfall intensity  
515 due to non-random microtopography (Langhans et al., 2013). The  $K_s$   
516 value used for the 'landscape' pixels, which covers the vast majority of  
517 the study area, was derived from rainfall experiments with an intensity  
518 of ca.  $45 \text{ mm h}^{-1}$  (Van den Putte et al., 2013). The design storm used in  
519 this study had an intensity lower than  $45 \text{ mm h}^{-1}$  for 92 % of the  
520 duration of the storm. Earlier research carried out on loam soils in  
521 Belgium has demonstrated a positive correlation between rainfall  
522 intensity and hydraulic conductivity (Langhans et al., 2010b). This  
523 implies an overestimation of the  $K_s$  value was made during the majority  
524 of the time covered by the modelled storm over our watershed and  
525 could have resulted in underestimated values of the runoff coefficient.  
526 Further,  $K_s$  has proven to be highly variable in the study region. Van den  
527 Putte et al. (2013) calculated a standard deviation of obtained hydraulic  
528 conductivity values of  $13.4 \text{ mm h}^{-1}$  for rainfall experiments carried out in  
529 the summer period. This high variability could have potentially led to an  
530 overestimation of  $K_s$  in our study area. Lastly, the observed runoff  
531 coefficients were derived for a catchment in which the average slope  
532 was 24 % higher compared to the study area used here. Catchments  
533 with higher slopes typically show higher values of the runoff coefficient  
534 (De Niel and Willems, 2019).

535 4.2. Impact of vLEs on runoff

536 The impact of the vLE density, connectivity, upslope area,  $K_s$  and  
537 Manning's  $n$  on the modelled total discharge volume, peak discharge  
538 rate and lag time was evaluated (Figure 4). A negative correlation was  
539 found between vLE density and discharge volume and peak discharge  
540 rate and a positive correlation between vLE density and lag time (Figure  
541 5). These findings are in line with previous research based on field  
542 experiments where higher vLE densities could be associated with  
543 increased infiltration, lower discharge volumes and peak discharge rates  
544 (Mérot, 1999; Viaud et al., 2005). Due to longer travel times and  
545 increased infiltration, the storm hydrograph is smoothed out (i.e., the  
546 lag time is longer and the peak discharge rate is reduced) in catchments  
547 with higher densities of vLEs (Mérot, 1999). This is also visualized in  
548 Figure 3 (scenarios 1, 2, 3): while  $K_s$ , Manning's  $n$  and  $\beta$  remain  
549 completely, or nearly constant across the 3 scenarios, an increase in vLE  
550 density  $\rho$  results in a lower discharge volume and peak discharge rate  
551 and a larger lag time, regardless of changes in upslope area. Therefore,  
552 it is recommended that the number of vLEs in the agricultural landscape  
553 does not further decrease, and better even, increases.  
554 Besides testing the impact of vLE density, we also looked at the effect of  
555 the connectivity of vLEs in the landscape by creating disconnections in  
556 the vLE objects. In reality, the connectivity of the vLE network in a  
557 landscape context is interrelated with the density of that vLE network:  
558 denser vLE networks also demonstrate a higher level of connectivity

559 (Burel and Baudry, 2012; Deckers et al., 2005). By keeping the density  
560 constant, we aimed at studying the effect of the connectivity  
561 independently of the vLE density. No strong impact of vLE connectivity  
562 on catchment runoff was found (Figure 4). The correlations between the  
563 level of connectivity and the discharge volume on the one hand and  
564 peak discharge rate, on the other hand, were both weakly negative and  
565 only present when the initial soil moisture content is low (Figure 6).  
566 The upslope area of the vLE objects showed to have a strong impact on  
567 the total discharge volume, the peak discharge rate and the lag time  
568 (Figure 4). In both dry and wet soils, we identified a negative  
569 relationship between the upslope area of the vLE objects and the  
570 discharge volume and peak discharge rate. This is in line with our  
571 expectations and indicates that vLE objects positioned on preferential  
572 flow paths downstream in the watershed can make a greater difference.  
573 When vLEs are associated with large values of the upslope area, a large  
574 proportion of the runoff will flow to the footprint of the vLE object. Due  
575 to the higher  $K_s$  values associated with vLE objects compared to the  
576 landscape pixels, more water can infiltrate. This is the concept behind  
577 the installation of grassed waterways where an increase of infiltration is  
578 achieved by decreasing runoff velocities through increasing the  
579 roughness and  $K_s$  (Evrard et al., 2008). The negative relationship  
580 between upslope area and peak discharge rate and the positive  
581 relationship with lag time can also be observed when the storm  
582 hydrographs were compared (Figure 3, scenarios 1 & 2): while all other

583 vLE characteristics remain constant, an increase in the upslope area  
584 results in a decrease of the discharge volume and peak discharge rate  
585 and an increase in the lag time.

586 vLE objects are associated with values of  $K_s$  that are up to 30 times  
587 higher compared to the surrounding agricultural land (Holden et al.,  
588 2019), which enhances infiltration. We could observe this effect in our  
589 modelling results where higher values of  $K_s$  associated with the vLEs  
590 result in a decrease of the discharge volume (Figure 8). We found an  
591 increase of the impact of the  $K_s$  value associated with the vLE object on  
592 discharge volume and peak discharge rate with increasing levels of soil  
593 wetness. This is contradictory to the findings of Hu et al. (2015) who  
594 concluded that the impact of  $K_s$  variability on runoff increased for lower  
595  $\theta_i$  values. Besides their higher  $K_s$  values, vLE objects are also  
596 characterized by higher values of Manning's  $n$  compared to surrounding  
597 agricultural land (Baartman et al., 2020; Richet et al., 2017). This  
598 increase in roughness is expected to reduce the velocity of the overland  
599 flow and thereby promoting infiltration and reducing the total runoff  
600 volume (Ferguson and Fenner, 2020). We did however not see this  
601 effect in our modelling results.

602 We applied the rainfall-runoff model to a relatively small watershed  
603 (26 ha) to quantify the impact of vLEs and their characteristics on runoff  
604 by using a design storm. In larger catchments, the river network  
605 configuration determines which areas of the catchment have the largest  
606 impact on the discharge peak while in small catchments this peak is

607 dominated by run-off from hillslopes in response to the storm (Dadson  
608 et al., 2017; Mérot, 1999). Therefore, it cannot be assumed that the  
609 effect of small-scale interventions can simply be extrapolated to  
610 estimate the combined effect at a larger scale (Dadson et al., 2017).

## 611 5. Conclusions

612 Using a distributed rainfall-runoff model, we demonstrated that total  
613 discharge volume, peak discharge rate and lag time to peak discharge  
614 are impacted more by the density of the vLE objects (positioned along  
615 the parcel boundaries) in the watershed and their upslope area in  
616 comparison to the vLE-connectivity, the saturated hydraulic conductivity  
617 of the soil underneath the vLE and the Manning's n coefficient  
618 associated with the vLE. The initial soil wetness level does not alter this  
619 relationship fundamentally.

620 Both for upslope area and vLE density, a negative correlation with the  
621 total discharge volume and peak discharge rate and a positive  
622 correlation with the lag time was demonstrated. The relationship is not  
623 linear though: e.g., a factor 8.7 increase in linear density leads to a  
624 reduction of some 20 % in discharge volume. The modelled impact of  
625 the  $K_s$  value associated with the soil underneath the vLE objects on the  
626 discharge volume and peak discharge rate was rather weak but  
627 increased with increasing wetness.  $K_s$  was shown to be negatively  
628 correlated with the discharge volume and peak discharge rate.

629 Connectivity and the Manning's n value associated with the vLE objects  
630 had a limited impact on the modelled discharge volume, peak discharge

631 rate and lag time. The connectivity of the vLE network had little impact  
632 on the total discharge volume, peak discharge rate and lag time  
633 We conclude that the more abundant the vLE along the agricultural  
634 parcel boundaries, the more rainfall is retained in the watershed.  
635 Hence, our modelling study confirms that vLEs contribute to a non-  
636 negligible extent to lowering the downstream flood risk and increasing  
637 the time lag to peak discharge, providing more opportunities for  
638 implementing punctual security measures.

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890  
891

892 **Appendix**

893 Overview of the 378 scenarios combining varying landscape patterns,  
894 different levels of vLE density, connectivity, and values of  $K_s$  and  
895 Manning's  $n$  associated with the vLE objects. For each configuration, 9  
896 unique combinations of  $K_s$  and Manning's  $n$  were modelled (indicated by  
897 the dotted lines).

898 **[Figure A.1]**

899 Table 1. Hydro-physical parameters used in the distributed rainfall-  
 900 runoff model.

Parameter	Unit	Value(s)
$\vartheta_i$	$\text{cm}^3 \text{cm}^{-3}$	0.02 – 0.155 – 0.29
Capillary pressure head at wetting front	mm	172.7
Landscape pixels		
$K_s$	$\text{mm hr}^{-1}$	19.2
Manning's $n$	$\text{s m}^{-1/3}$	0.08
vLE pixels		
$K_s$	$\text{mm hr}^{-1}$	20 – 51.2 – 102.4
Manning's $n$	$\text{s m}^{-1/3}$	0.30 – 0.43 – 0.55

901

902 Table 2. Description of four distinct model scenarios and their associated  
 903 output for  $\vartheta_i = 0.29 \text{ m}^3 \text{ m}^{-3}$ .

scenario	vLE characteristics				Output variables			
	density ( $\text{m ha}^{-1}$ )	$K_s$ ( $\text{mm h}^{-1}$ )	Manning's $n$ ( $\text{s m}^{-1/3}$ )	$\beta$ (-)	Upslope area ( $\text{ha m}^{-1}$ vLE)	Discharge volume ( $\text{m}^3$ )	Peak discharge rate ( $\text{m}^3 \text{s}^{-1}$ )	Lag time (s)
1	10	102.4	0.43	0.55	8.0	94	0.067	623
2	10	102.4	0.43	0.55	1.3	126	0.086	590
3	87	102.4	0.43	0.58	4.0	66	0.047	706
4	87	20	0.43	0.58	4.0	109	0.071	693

904

905 Table 3. Accuracy of the random forest regression models applied on the  
 906 testing set of vLE-scenarios predicting discharge volume, peak discharge rate  
 907 and lag time for  $\vartheta_i = 0.02 \text{ m}^3 \text{ m}^{-3}$ ,  $\vartheta_i = 0.155 \text{ m}^3 \text{ m}^{-3}$  and  $\vartheta_i = 0.29 \text{ m}^3 \text{ m}^{-3}$ .

	Discharge volume ( $\text{m}^3$ )		Peak discharge rate ( $\text{m}^3 \text{s}^{-1}$ )		Lag time (s)	
	RMSE	R <sup>2</sup>	RMSE	R <sup>2</sup>	RMSE	R <sup>2</sup>
$\vartheta_i = 0.02 \text{ m}^3/\text{m}^{-3}$	8.11E-02	0.98	2.79E-04	0.97	9.55E+00	0.84
$\vartheta_i = 0.155 \text{ m}^3/\text{m}^{-3}$	5.36E-01	0.98	6.70E-04	0.96	2.91E+01	0.84
$\vartheta_i = 0.29 \text{ m}^3/\text{m}^{-3}$	2.24E+00	0.98	1.35E-03	0.98	5.45E+01	0.33

908

909 Table 4. Summary of the Kruskal-Wallis test results for different levels of initial  
 910 soil moisture content ( $\vartheta_i$ ).

Feature	Output variable	$\chi^2$ (P-value)		
		$\vartheta_i = 0.02 \text{ m}^3 \text{ m}^{-3}$	$\vartheta_i = 0.155 \text{ m}^3 \text{ m}^{-3}$	$\vartheta_i = 0.29 \text{ m}^3 \text{ m}^{-3}$
vLE density	Discharge volume	65.179 (7.024E-15)**	140.580 (< 2.2E-16)**	171.530 (< 2.2E-16)**
	Peak discharge rate	45.411 (1.378E-10)**	147.760 (< 2.2E-16)**	136.070 (< 2.2E-16)**
	Lag time	38.516 (4.330E-09)**	5.551 (0.062)	32.745 (7.754E-08)**
Connectivity	Discharge volume	4.097 (0.043)*	1.324 (0.250)	2.231 (0.135)
	Peak discharge rate	5.751 (0.016)*	0.418 (0.518)	3.359 (0.067)
	Lag time	0.224 (0.636)	1.546 (0.214)	1.514 (0.219)
Upslope area	Discharge volume	71.307 (2.24E-15)**	32.970 (3.267E-07)**	136.04 (< 2.2E-16)**
	Peak discharge rate	72.101 (1.514E-15)**	32.419 (4.27E-07)**	137.97 (< 2.2E-16)**
	Lag time	41.263 (5.751E-09)**	5.379 (0.146)	199.52 (< 2.2E-16)**
$K_s$	Discharge volume	1.231 (0.540)	14.951 (5.668E-04)**	69.388 (8.561E-16)**
	Peak discharge rate	0.178 (0.915)	10.444 (5.395E-03)**	18.104 (1.172E-04)**
	Lag time	0.902 (0.637)	46.442 (8.229E-11)**	0.610 (0.737)
Manning's n	Discharge volume	2.026 (0.363)	0.828 (0.661)	1.878 (0.391)
	Peak discharge rate	0.011 (0.995)	1.101 (0.577)	0.211 (0.900)
	Lag time	1.509 (0.470)	0.327 (0.849)	2.028 (0.363)

911 Statistical significance of the relationship is noted as: \* significant at  $\leq 0.05$ , and \*\* at

912  $\leq 0.01$ .

Figure 1

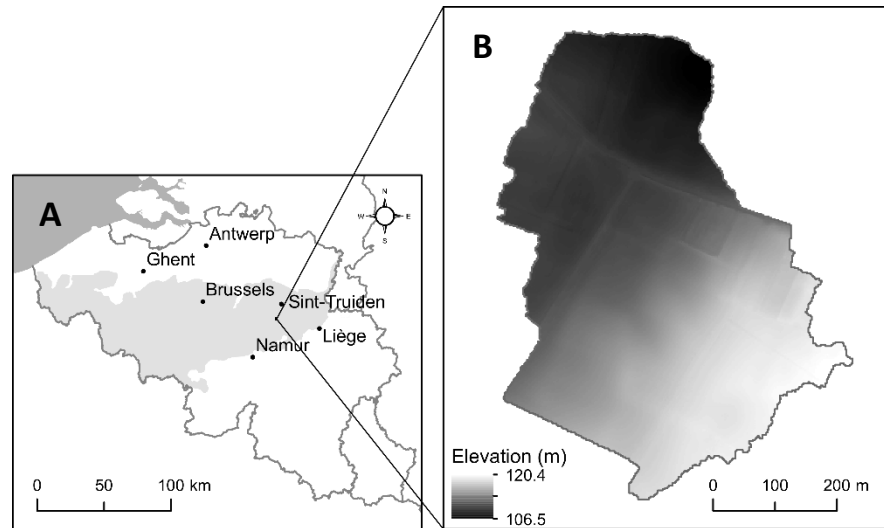


Figure 2



Figure 3

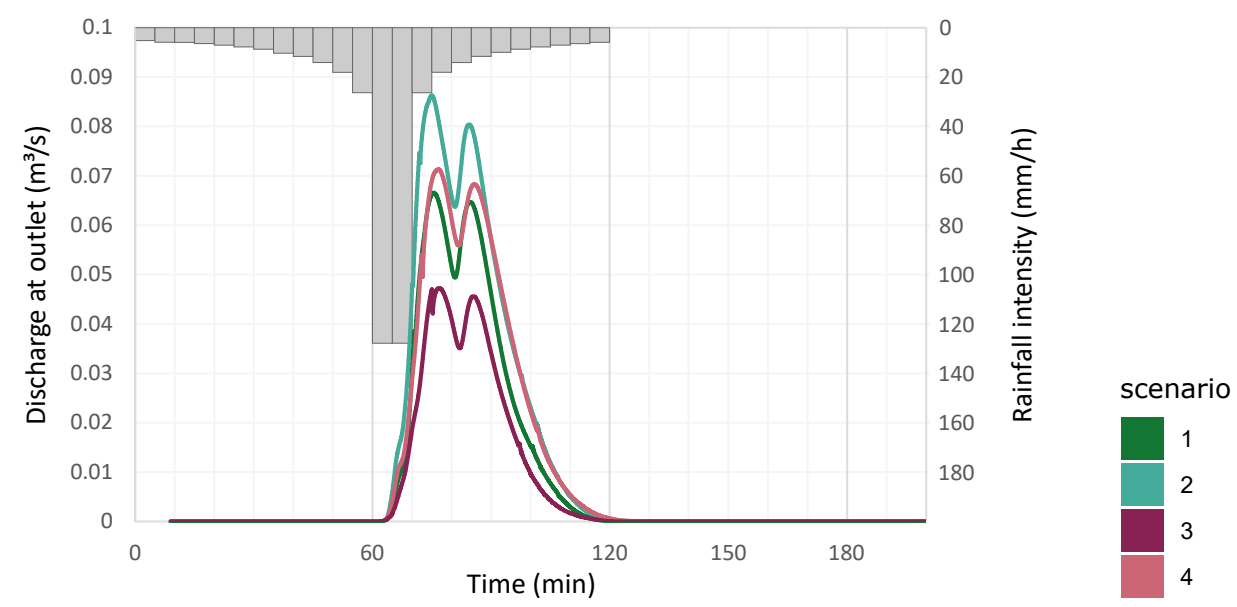


Figure 4

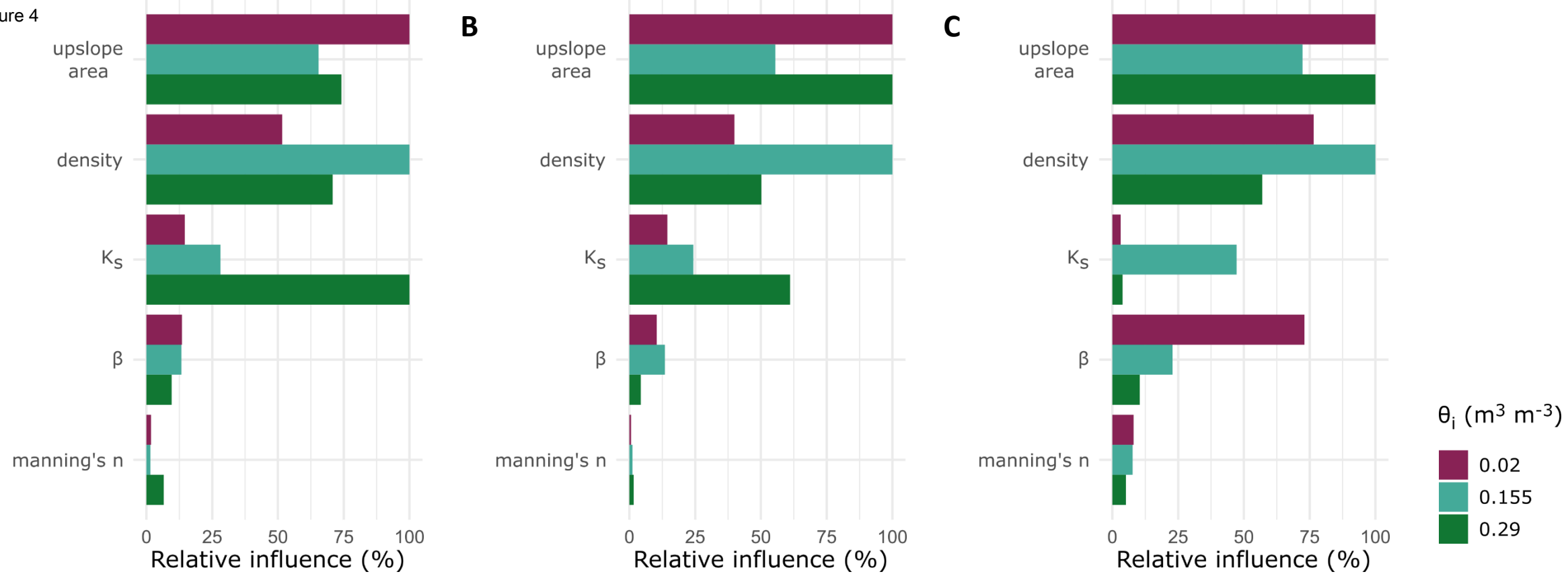




Figure 5

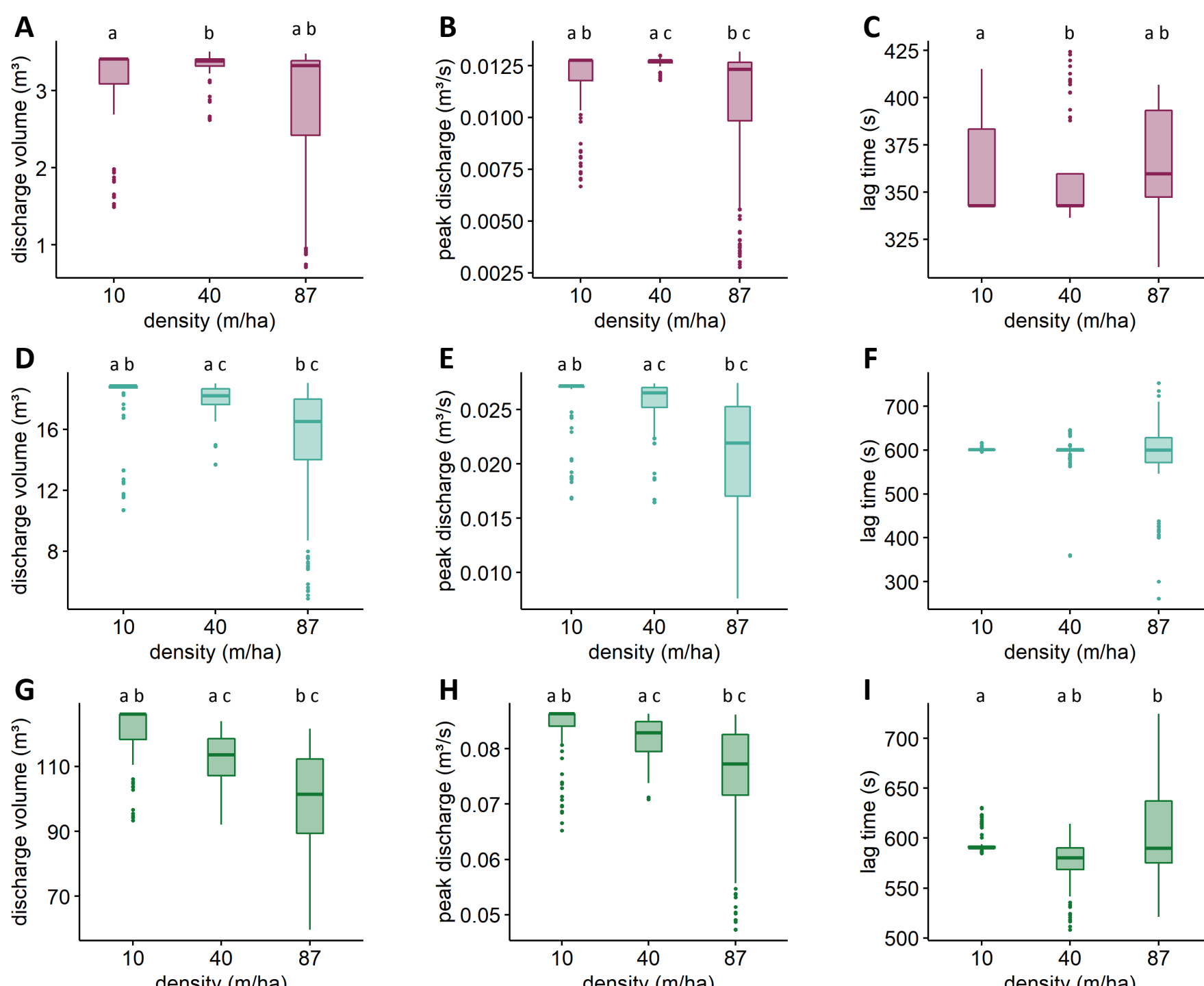


Figure 6

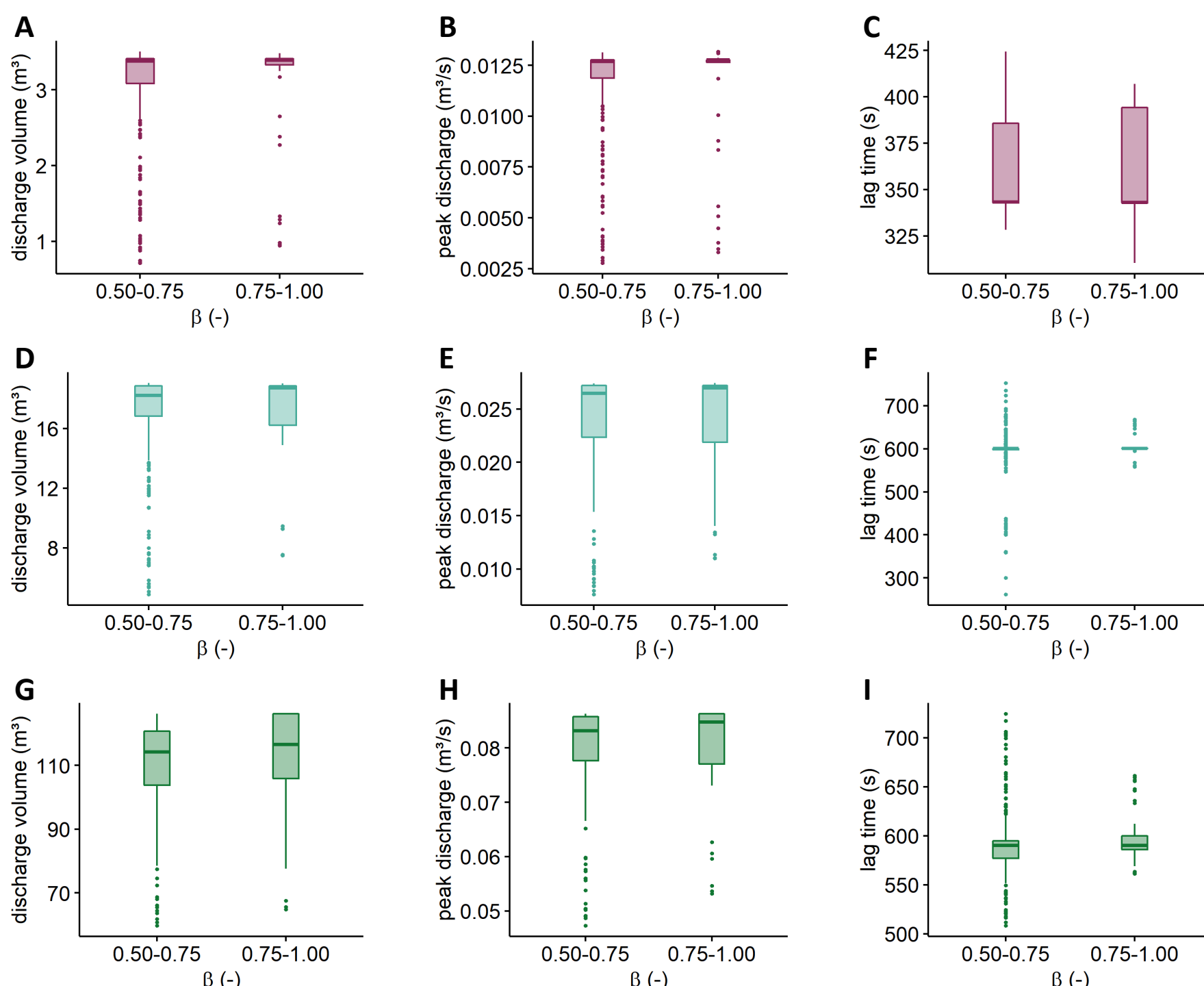


Figure 7

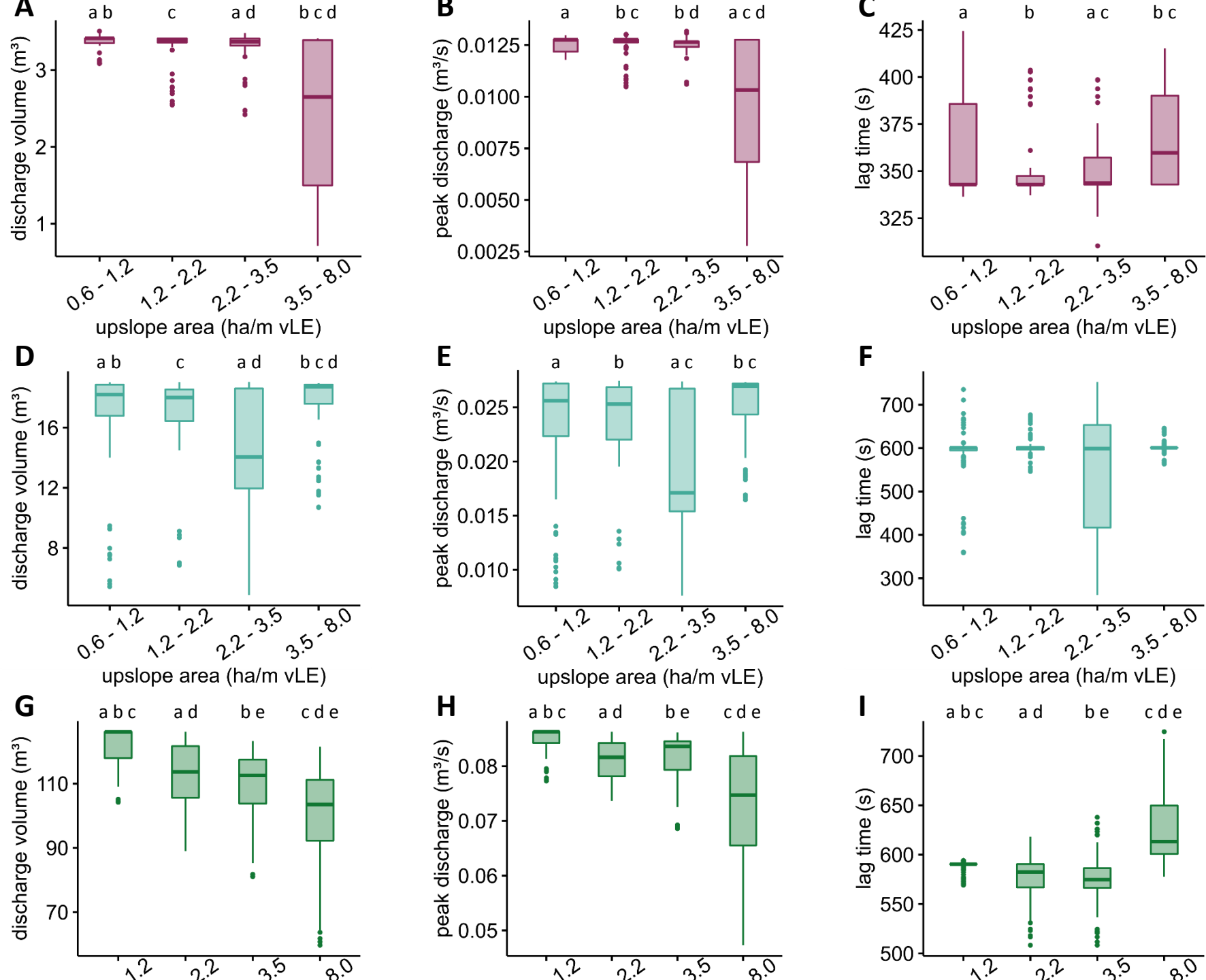


Figure 8

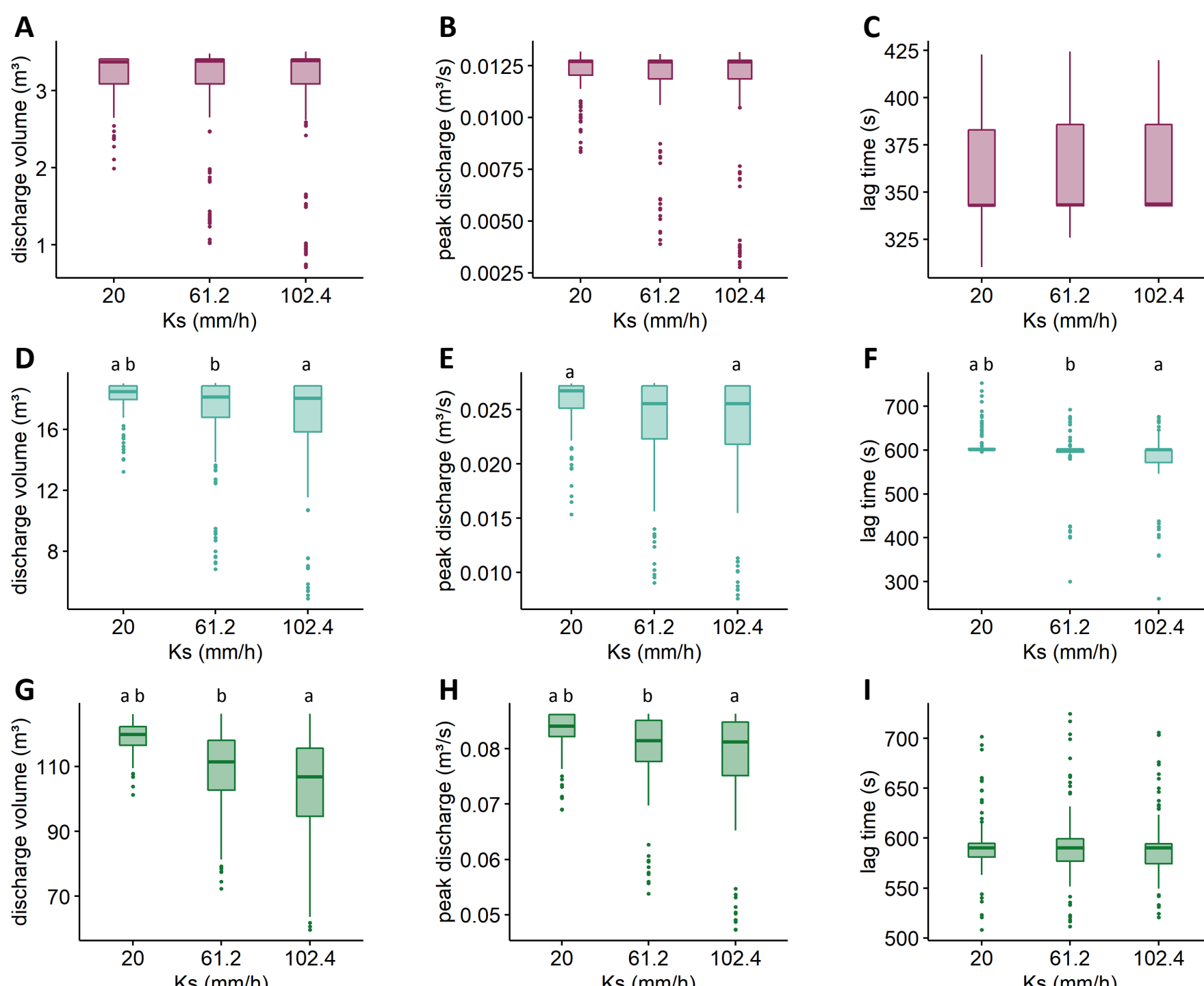


Figure 9

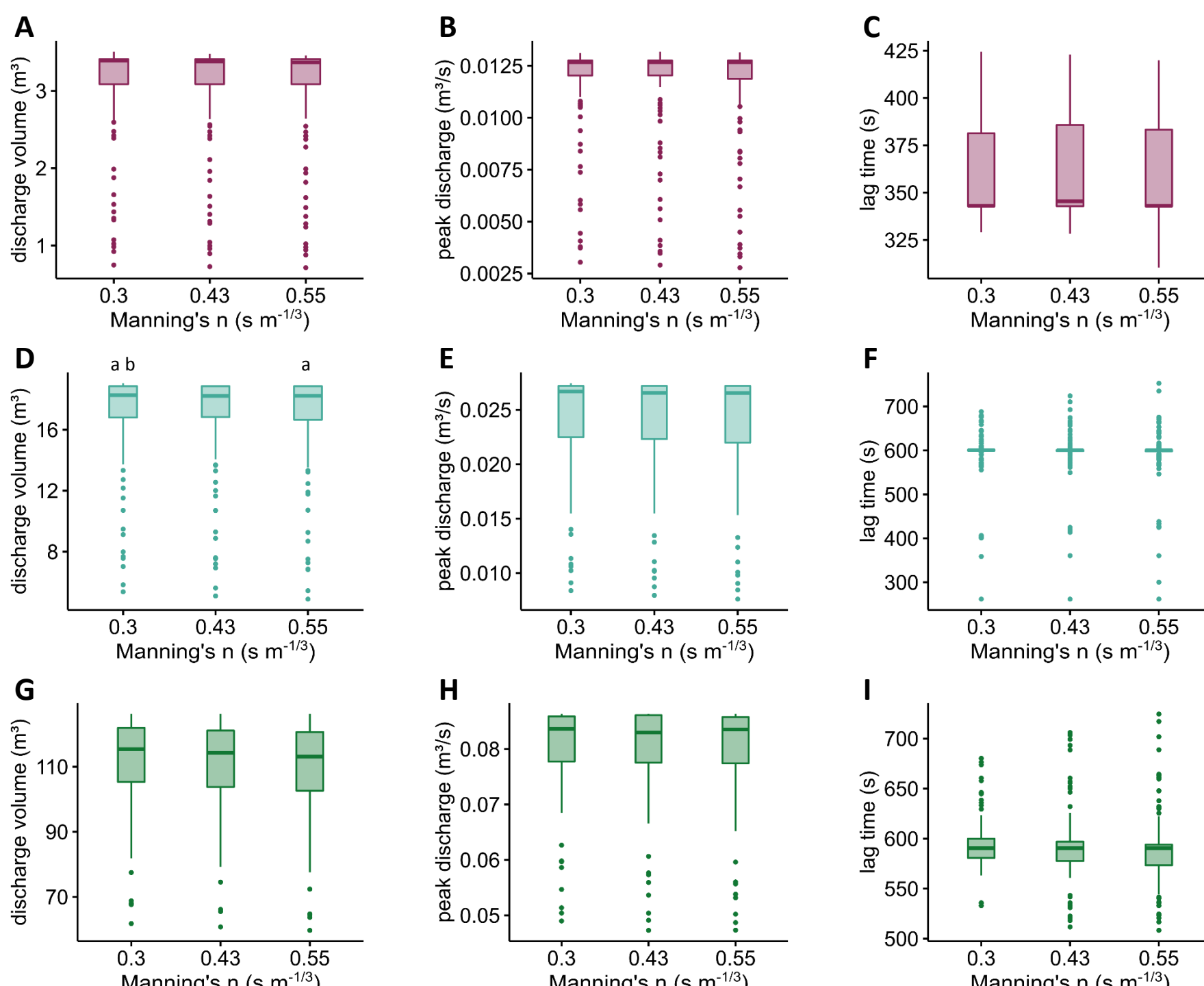
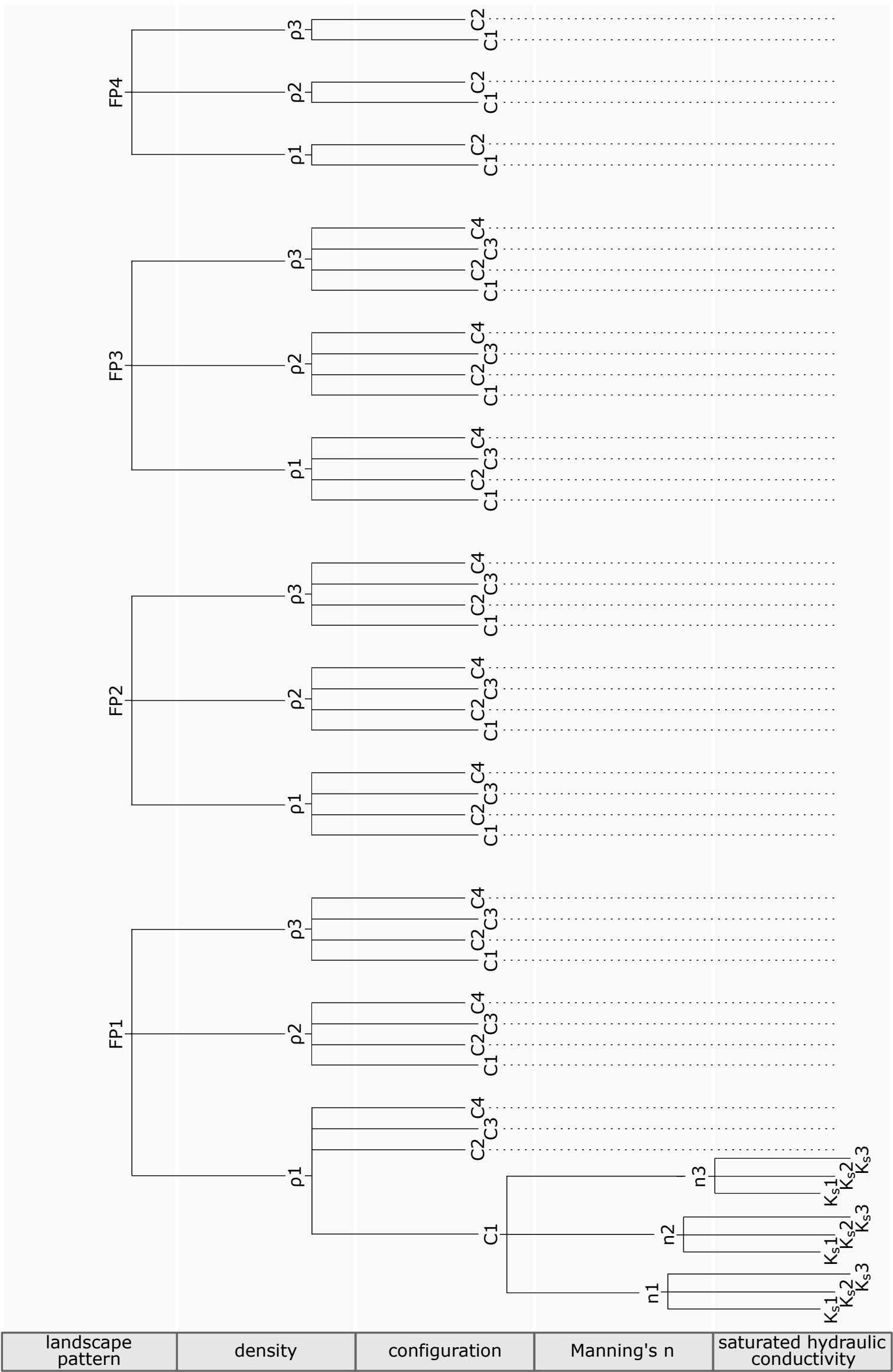


Figure A1



landscape pattern	density	configuration	Manning's n	saturated hydraulic conductivity
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**Figure 1.** (A) Location of the watershed in the Belgian loess belt (grey) based on Evrard et al. (2007b). (B) Watershed elevation model.

**Figure 2.** Landscape patterns in (A) FP1, (B) FP2, (C) FP3, and (D) FP4 considered in the watershed.

**Figure 3.** Discharge at the watershed outlet for four different scenarios detailed in Table 2 and the associated hyetograph for  $\vartheta_i = 0.29 \text{ m}^3 \text{ m}^{-3}$ .

**Figure 4.** Global variable importance plots for the prediction of (A) the total discharge volume, (B) the peak discharge rate and (C) the lag time for  $\vartheta_i = 0.02 \text{ m}^3 \text{ m}^{-3}$  (dry soils, purple),  $\vartheta_i = 0.155 \text{ m}^3 \text{ m}^{-3}$  (intermediate wet soils, light green) and  $\vartheta_i = 0.29 \text{ m}^3 \text{ m}^{-3}$  (wet soils, dark green). The five considered variables are 'upslope area': the upslope area of the vLE objects, 'density': the density of the vLE objects in the watershed, ' $K_s$ ': the saturated hydraulic conductivity associated with the soil underneath the vLE objects; ' $\beta$ ': the beta-connectivity index of the vLE objects in the watershed, and 'Manning's  $n$ ': the Manning's roughness coefficient associated with the vLE objects. The different shadings indicate different levels of initial soil moisture content.

**Figure 5.** Impact of vLE density on total discharge volume, peak discharge rate and lag time for  $\vartheta_i = 0.02 \text{ m}^3 \text{ m}^{-3}$  (A, B & C),  $\vartheta_i = 0.155 \text{ m}^3 \text{ m}^{-3}$  (D, E & F) and  $\vartheta_i = 0.29 \text{ m}^3 \text{ m}^{-3}$  (G, H & I). Lowercase letters above the boxplots show the results of the Dunn's test, with statistically similar (P-value < 0.05) levels grouped by the same letter.

**Figure 6.** Impact of connectivity expressed as beta connectivity index ( $\theta$ ) on total discharge volume, peak discharge rate and lag time for  $\vartheta_i = 0.02 \text{ m}^3 \text{ m}^{-3}$  (A, B & C),  $\vartheta_i = 0.155 \text{ m}^3 \text{ m}^{-3}$  (D, E & F) and  $\vartheta_i = 0.29 \text{ m}^3 \text{ m}^{-3}$  (G, H & I).

**Figure 7.** Impact of upslope area of the vLEs on total discharge volume, peak discharge rate and lag time for  $\vartheta_i = 0.02 \text{ m}^3 \text{ m}^{-3}$  (A, B & C),  $\vartheta_i = 0.155 \text{ m}^3 \text{ m}^{-3}$  (D, E & F) and  $\vartheta_i = 0.29 \text{ m}^3 \text{ m}^{-3}$  (G, H & I).

**Figure 8.** Impact of the saturated hydraulic conductivity ( $K_s$ ) associated with the vLE segments on total discharge volume, peak discharge rate and lag time for  $\vartheta_i = 0.02 \text{ m}^3 \text{ m}^{-3}$  (A, B & C),  $\vartheta_i = 0.155 \text{ m}^3 \text{ m}^{-3}$  (D, E & F) and  $\vartheta_i = 0.29 \text{ m}^3 \text{ m}^{-3}$  (G, H & I). Lowercase letters above the boxplots show the results of the Dunn's test, with statistically similar (P-value < 0.05) levels grouped by the same letter.

**Figure 9.** Impact of the Manning's roughness coefficient ( $n$ ) associated with the vLE segments on total discharge volume, peak discharge rate and lag time for  $\vartheta_i = 0.02 \text{ m}^3 \text{ m}^{-3}$  (A, B & C),  $\vartheta_i = 0.155 \text{ m}^3 \text{ m}^{-3}$  (D, E & F) and  $\vartheta_i = 0.29 \text{ m}^3 \text{ m}^{-3}$  (G, H & I).

**Figure A.1** Overview of the 378 scenarios used in this study.